

#### United States Department of Agriculture

Natural Subject: ENG - Soil Mechanics Report Date: MAY 2 7 2015

Resources
Conservation
Service

Plum Creek # 12
Hays County, Texas

National Design,
Construction,

Mechanics
To: John Mueller, P.E.

State Conservation Engineer

File Code: 210-22

Job No: 7562

501 W. Felix, NRCS, Temple, TX Bldg. 23

Texas 76115

Phone:
817.509.3204

Fort Worth,

and Soil

### INTRODUCTION

Plum Creek # 12 is an existing embankment dam located in Hays County, Texas. Seven boreholes (Number 207 through 213) were advanced to depths ranging from 5.0 to 17.5 feet along the northwest, inside edge of the auxiliary spillway.

Nine Shelby tubes of undisturbed field samples were delivered from five of the boreholes to the Fort Worth Soil Mechanics Laboratory (SML) for testing. Requested testing included: index, dry unit weight, and unconfined compressive strength. The purpose of this report is to provide the results of the unconfined compressive strength testing (qu) which will be utilized to determine the headcut erodibility index (Kh).

#### INTERPRETATION AND DISCUSSION OF DATA

### **Index Tests and Water Content**

Index testing was completed on thirteen soil samples and the majority classified as fat clay (CH) according to the Unified Soil Classification System (USCS). Three of the soil samples from shallow depths in the boreholes classified as elastic silt (MH; F15-314 / 207.1), gravelly elastic silt (MH; F15-316 / 208.1), and sandy elastic silt with gravel (MH; F15-317 / 208.2). One sample classified as lean clay (CL; F15-325 / 210.2) and one sample classified as clayey gravel with sand (GC; F15-318 / 208.3). The Liquid Limit (LL) values varied from 38 to 65 and Plasticity Index (PI) values ranged from 18 to 34.

Index properties of the samples are shown in Attachment 1 on form SCS-ENG-354.

#### **Dispersion Tests**

Double Hydrometer and Crumb tests were performed on all of the soil samples. Double Hydrometer test results less than about 60 indicate that dispersion is not a problem, and this was the case for all of the soil samples tested. Crumb test results of 1 indicate that dispersion is not

present or is minimal, but results of 3 or 4 are positive indicators that clays are dispersive. The testing of soil samples from this area of the project site did not indicate that these samples have dispersive clay characteristics.

## <u>Unconfined Compressive Strength (qu) Tests</u>

The Unconfined Compressive Strength of Cohesive Soil test (ASTM D2166) was performed on eight soil samples at the Lincoln Soil Mechanics Laboratory in Nebraska. The samples were sheared at or below their natural moisture content and associated saturation level. The saturation level of the samples ranged from 74.3 % to 99.7 %. However, the majority of the samples had a saturation level of 88 % or more. The sample with the lowest saturation level was from a depth of less than four feet.

All eight of soil samples were sheared at their extruded diameter (+2.7 inch diameter), and were in compliance with the ASTM D2166's height-to-diameter ratio.

A summary of the peak compressive stresses is provided below and the full test reports are provided as Attachment 2.

Lab Sample No.	Field Sample No.	Depth (ft)	Natural Moisture Content (%)	Percent Saturation (%)	qu (psf)
315	207.2	5.0 - 7.5	15.3	92.3	11,830*
319	208.4	10.0 - 11.5	18.3	94.0	8,130
320	208.5	15.0 - 17.5	17.9	90.8	4,165
321	209.1	2.5 - 4.0	20.1	74.3	7,585
323	209.3	10.0 - 12.5	19.5	99.7	10,135
324	210.1	2.5 - 5.0	23.7	88.7	5,830
325	210.2	10.0 - 12.5	19.7	96.5	9,285
326	211.1	7.5 - 10.0	19.3	95.8	8,970

Note: \* The load cell maxed out before this sample failed

### CONCLUSIONS AND RECOMMENDATIONS

The following are conclusions regarding the results of the testing:

- 1) The results of the unconfined compressive strength tests ranged from 4,165 to 11,830 psf.
- 2) The recorded q<sub>u</sub> values are peak stresses.
- 3) Consideration should be given to whether the unconfined compressive strength of the sample is appropriate for calculation of the headcut erodibility index (Kh) considering the soil in the spillway will fail in shear, when it experiences water flow.

- 4) The majority of the soil samples were fat clay (CH). However, BH 208 (the upper 10ft) classified as MH / GC and as the hatched area shows on the borehole location map (Figure 1) this may have been fill material during construction. The strength increased with depth in BH 209 and BH 210, but this was not the case at BH 208. Because of this anomaly at BH 208 extra analysis should be taken in developing the Kh for the entire auxiliary spillway and a determination of whether this borehole along with the elastic silt identified in BH 207 represents an area that should be treated with caution. Furthermore, this could be a significant anomaly within the auxiliary spillway and a critical parameter that requires added attention, engineering judgement, and sensitivity analysis.
- 5) There was significantly more of a sand portion in the soil samples identified as elastic silt when compared to the soil samples classified as fat clay.

If you would like to discuss this report or if you need to request further testing, please contact me at (817) 509-3204.

Prepared by:

STEVEN GARNER, P.E.

Civil Engineer

NRCS, Fort Worth, TX

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Concurred by:

Head, Soil Mechanics Laboratory

NRCS, Fort Worth, TX

PHILLIP T. RIPPÉ, P.E

### Figures:

1. Borehole Locations

#### Attachments:

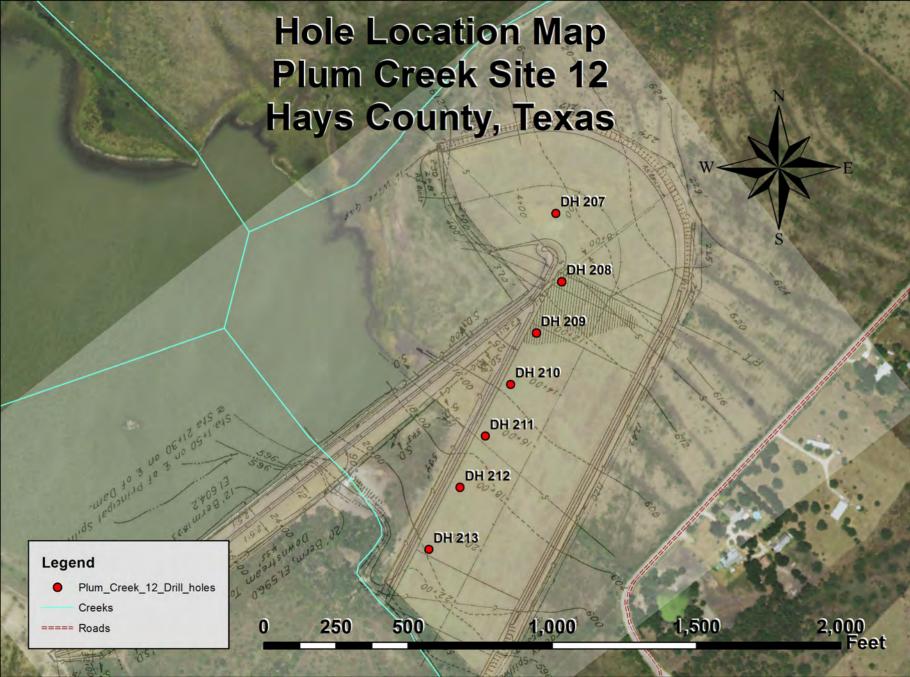
- 1. Form SCS-ENG-354, Soil Mechanics Laboratory Test Data, 1 sheet
- 2. Unconfined Compression Test Report, 8 sheets

cc: (electronically distributed)

John Hebrik, State Design Engineer, NRCS, Temple, TX Stephen Reinsch, Co-Director, NDCSMC, NRCS, Lincoln, NE Noller Herbert, Director, CED, NRCS, Washington, DC

# Figure 1

Borehole Locations, 1 sheet



## Attachment 1

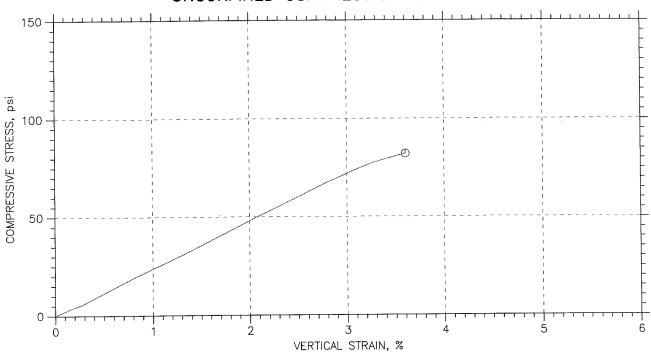
NRCS-ENG-354, Soil Mechanics Data, 1 sheet

Sheet \_1\_\_ of \_1\_\_

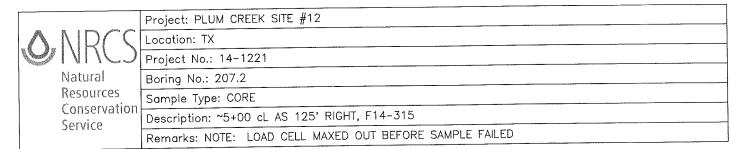
ī																										Т	T		_			Job	No.	7562	ı		
	8/20	)/14	TX	WF-07				Gra	ain Size	Distrib	Me oution Ex	echani xpress	cal An	alysis Percent	Finer	by Dry	Weight					tterber Limits	_	ation	%	(%) €	Weigh	tion		Dispers	ion	Moi	sture-D	Density			
	Lab. Sample	Field Sample	Plum Creek Site # 12				Fines					San	d					Grav	el			Limits		assific	Soluble Salts	Natural Moisture (%)	Natural Dry Unit V (gm/cm <sup>3</sup> )	Percent Saturation	ter				ASTM D	698 G <sub>S</sub>	G <sub>M</sub>	% Absorp-	рН
	No.	No.	7562 Hays Co.	Depth (ft) Sample Type					#200	#140	#60	#40	#20	#10	#4	3/8"	1/2" 3	3/4"	1" 1	1/2" 3		L. P	P.I.	ied Cla	əlqnlc	ıral Mc	al Dry (gm/	cent 8		Crumb 7		Ma		w <sub>0</sub> %	O <sub>M</sub>	tion	Pii
			Location and Description	1	0.002 mm		0.02 mm	0.05 mm	0.074 mm	0.105 mm	0.250	0.42	0.84 mm	2.0	4.76	9.525	12.7 1	9.05	25.4	38.1 76	5.2	'		Unifi	Й	Natu	Natura	Per	Dou	1 Hr   4	Hr	(b	cf)	, 70			
F14				0.0 -	48			86					94				İ	100				5 2	29	МН	<.5	19.1		Ì	8	1	1			2.68			
	314	207.1	Sta 5+00 CL AS 125 ft right	t 0.0 Small																															<u> </u>		
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	315	207.2	Sta 5+00 CL AS 125 ft right	7.5 Undist.																		_						<u> </u>							ŀ	<del> </del>	
				0.0 -	35	44	57	68	70	71	72	73	73	75	83	84	84	89	92	100	6	1 2	28	МН	<.5	16.8			8	1	1			2.56			
	316	208.1	Sta 11+25 CL AS 175 ft righ	1.5 Undist.																						1		<u> </u>								<del> </del>	
				5.0 - Small	34	43	55	68	69	69	69	70	72	74	85	95	100				5	9 2	28	МН	<.5	20.1			19	1	1			2.65			$\square$
	317	208.2	Sta 11+25 CL AS 175 ft righ	1 5.5 Small			00	•				. •					100								7,0	20.1					•			2.00	<u> </u>	<u> </u>	
					9	13	18	21	21	21	23	25	28	32	50	70	80	93	98	100	3	8 1	8	GC	<.5	8.8			15	1	1			2.69		<u> </u>	
	318	208.3	Sta 11+25 CL AS 175 ft righ	t 6 - 9 Small										-		. •					<u> </u>				7,0	0.0					•			2.00	<u>.</u>		
				10.0	48	71	90	97	100												5	6 3	32	СН	<.5	19.4			28	1	1			2.68		<u> </u>	
	319	208.4	Sta 11+25 CL AS 175 ft righ	nt 10.0 - Undist.		' '	00	01	100															011	7.0	10.1				'	<u> </u>			2.00	<u> </u>	<u> </u>	
				15.0	47	66	85	96	98		_	_	_ [	100							5	3 3	81	СН	<.5	19.5			32	1	1			2.68		<del> </del>	
	320	208.5	Sta 11+25 CL AS 175 ft righ	nt 15.0 - Undist.			00	00						100										011	7.0	10.0				'	<u> </u>			2.00	<u> </u>	<u> </u>	
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	321	209.1	Sta 13+25 CL AS 175 ft righ	at $\begin{vmatrix} 2.5 - \\ 4.0 \end{vmatrix}$ Undist.		00	70	31	-					100										011	7.0	21.1				'	<u>'                                       </u>	<u> </u>		2.04	<u> </u>	<u> </u>	
				7.5	51	73	86	97	99		_	_	_	100							5	3 3	32	СН	<.5	20.5			21	1	1			2.70			
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				10.0	47	66	85	94	96	-	-	_	_ [	100							5	3 3	32	СН	<.5	15.5			29	1	1			2.67	!		
	323	209.3	Sta 13+25 CL AS 175 ft righ	nt 10.0 - Undist.		00	00	<b>5</b> 4	30					100							<b>_</b>	0 0		011	7.0	10.0				'	<u>'</u>			2.07		<u> </u>	
				0.5	47	59	78	94	96		_	_	_	100							6	0 3	<u>.</u>	СН	<.5	19.2			12	1	1			2.64		<u> </u>	
	324	210.1	Sta 15+25 CL AS 175 ft righ	t 2.5 - Undist.		03	70	7	30	-			·	100								. 3	, ,	J11	,	13.2			12	'	'			2.04	<u> </u>	<u> </u>	
				10.0	40	58	72	Q1	96	_	_	_	_	100							1	7 2	26	CL	<.5	18.3			14	1	1			2.66		<del>                                     </del>	$\vdash$
	325	210.2	Sta 15+25 CL AS 175 ft righ	10.0 - Undist.		50	70	31	30	-			·	100							+	.   2	-	OL	,	10.5			14	'	'			2.00		<u> </u>	$\vdash$
				7.5	12	62	75	94	96		_		_	100								0 2	00	СН	<.5	14.3			15	1	1			2.69	<u> </u>		$\vdash$
	326	211.1	Sta 17+25 CL AS 175 ft righ	7.5 - Undist.	. 43	02	73	54	90	-		-		100							3	2	.0	ОΠ	<.5	14.3			10		1			2.09		<u> </u>	
																								į								i			<u> </u>	<u> </u>	

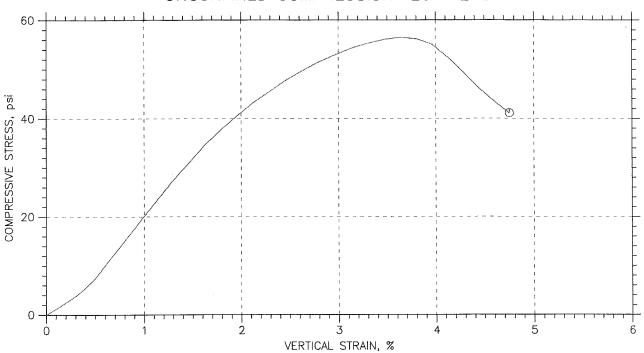
## Attachment 2

Unconfined Compression Test Report, 8 sheets

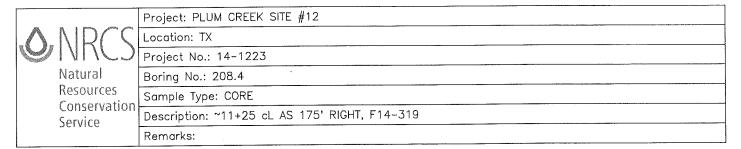


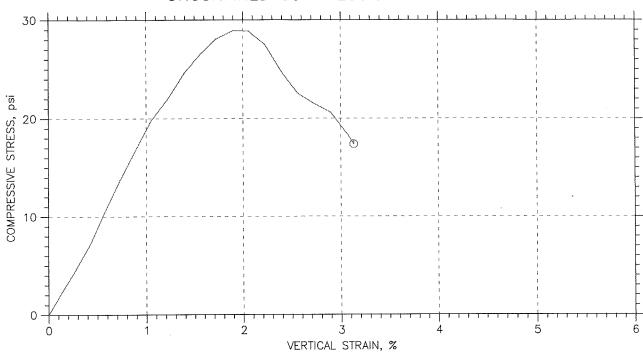
Sy	mbol	0		,
Те	st No.	1		
	Diameter, in	2.734 ,		
	Height, in	5.952		
ial	Water Content, %	15.31		
Initial	Dry Density, pcf	116.1		
	Saturation, %	92.25		
	Void Ratio	0.447		
Ur	confined Compressive Strength, psi	82.15	-	
Ur	ndrained Shear Strength, psi	41.07		
Tir	ne to Failure, min	3.6819		
St	rain Rate, %/min	1 '		
М	easured Specific Gravity	2.69 ′		
Li	quid Limit			
PI	astic Limit			
PI	asticity Index			
-	illure Sketch			



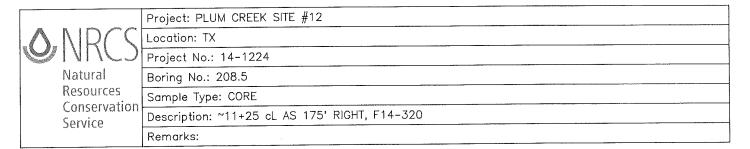


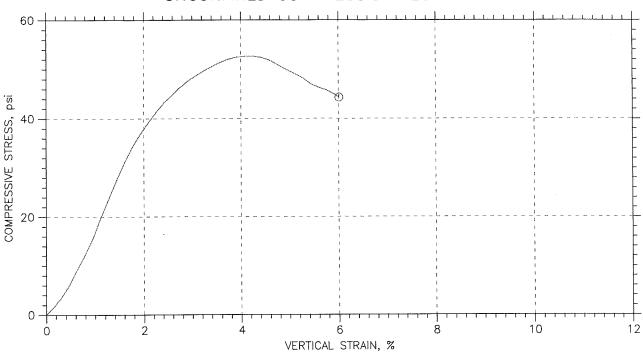
Sy	mbol	0		
Те	st No.	1		
	Diameter, in	2.754		
	Height, in	5.97 /		
ig	Water Content, %	18.29		
Initial	Dry Density, pcf	109.9		
	Saturation, %	93.97		
	Void Ratio	0.522		
Ur	nconfined Compressive Strength, psi	56.47		
Ur	ndrained Shear Strength, psi	28.24		
Tii	ne to Failure, min	3.6694		
St	rain Rate, %/min	1 ,		
М	easured Specific Gravity	2.68		
Li	guid Limit			
PI	astic Limit			
PI	asticity Index			
Fo	ilure Sketch			: f : ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;





Sy	mbol	0		
Te	st No.	1		
	Diameter, in	2.771 ,		
	Height, in	5.876 .		
<u>ā</u> .	Water Content, %	17.88		
Initial	Dry Density, pcf	109.5		
	Saturation, %	90.79		
	Void Ratio	0.528		
U	nconfined Compressive Strength, psi	28.93		
U	ndrained Shear Strength, psi	14.47		
Ti	me to Failure, min	2.0022		
St	rain Rate, %/min	1 .		
М	easured Specific Gravity	2.68		
Li	quid Limit			
P	astic Limit			
P	asticity Index			
F	ailure Sketch			- 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1

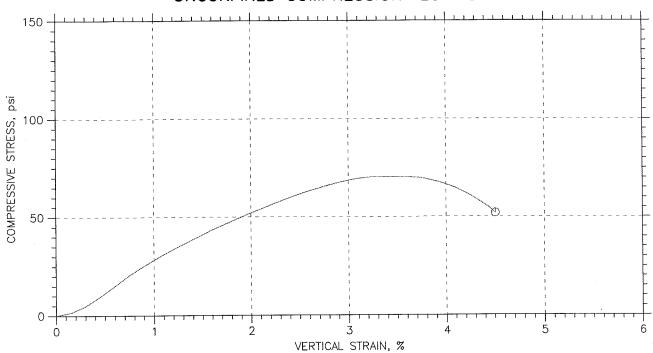




Sy	mbol	Ф		
Те	st No.	1		
	Diameter, in	2.724		
	Height, in	6.015		
<u>a</u>	Water Content, %	20.09		
Initial	Dry Density, pcf	96.18		
	Saturation, %	74.33		
	Void Ratio	0.714		
Ui	nconfined Compressive Strength, psi	52.66		
Ui	ndrained Shear Strength, psi	26.33		
Tii	me to Failure, min	4.3359		
St	rain Rate, %/min	1 /		
М	easured Specific Gravity	2.64		
Li,	quid Limit			
PI	astic Limit			
PI	asticity Index			
Fo	illure Sketch			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

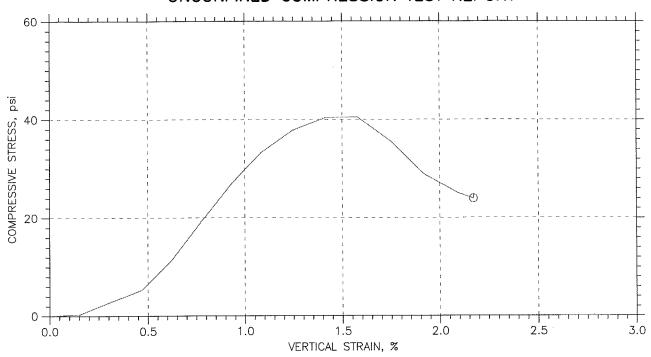


	Project: PLUM CREEK SITE #12
	Location: TX
)	Project No.: 14-1225
	Boring No.: 209.1
. 300	Sample Type: CORE
)N	Description: ~13+25 cL AS 175' RIGHT, F14-321
	Remarks:

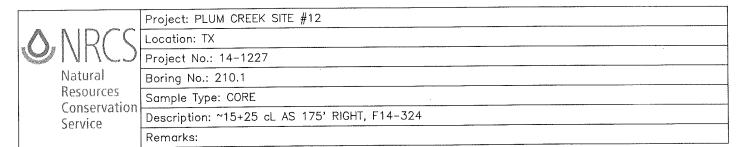


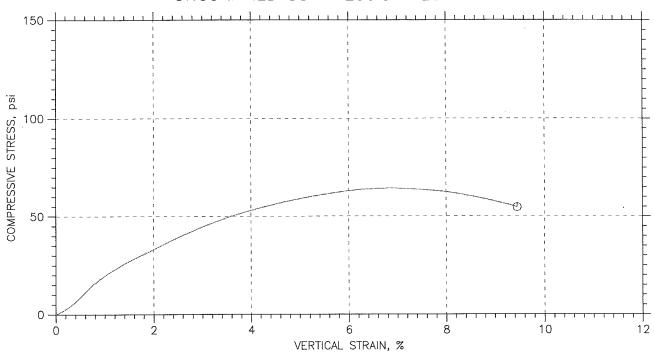
Sy	mbol	0		
Те	st No.	1		
	Diameter, in	2.769		
	Height, in	5.897 ,		
<u>ā</u> .	Water Content, %	19.51		
Initial	Dry Density, pcf	109.5		
	Saturation, %	99.67		
	Void Ratio	0.523		
Ur	nconfined Compressive Strength, psi	70.38		
Ur	ndrained Shear Strength, psi	35.19		
Tir	me to Failure, min	3.6684		
St	rain Rate, %/min	1 ,		
М	easured Specific Gravity	2.67		
Li	quid Limit			
PI	astic Limit	<b></b>		
PI	asticity Index			
Fo	silure Sketch			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

	Project: PLUM CREEK SITE #12
A NIDCC	Location: TX
RAININO	Project No.: 14-1226
Natural	Boring No.: 209.3
	Sample Type: CORE
Conservation Service	Description: ~13+25 cL AS 175' RIGHT, F14-323
JC1 V1CC	Remarks:

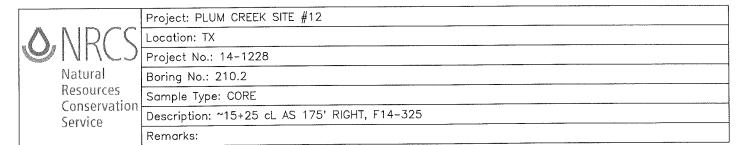


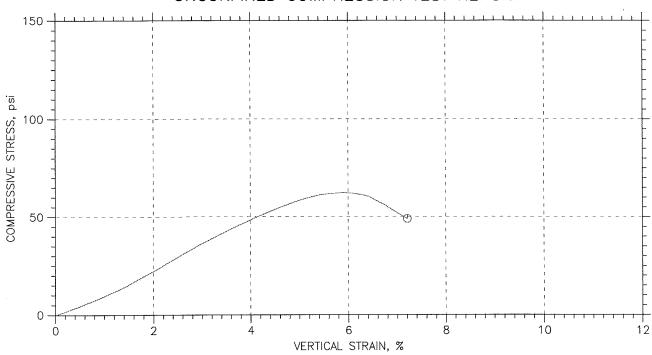
Sy	mbol	Ф		
Те	st No.	1		
	Diameter, in	2.764		
	Height, in	5.944 ,		
<u>a</u> .	Water Content, %	23.67 ,		
Initial	Dry Density, pcf	96.67 ,		
	Saturation, %	88.65		
	Void Ratio	0.705		
Uı	nconfined Compressive Strength, psi	40.48		
Uı	ndrained Shear Strength, psi	20.24		
Ti	me to Failure, min	1.6706		
St	rain Rate, %/min	1 '		
М	easured Specific Gravity	2.64 /		
Li	quid Limit			
PI	astic Limit			
PI	asticity Index			
Fo	silure Sketch			: : : : : : : : : : : : : : : : : : :





Sy	ymbol	0		
Те	est No.	1		
	Diameter, in	2.767		
	Height, in	5.897		
<u>ia</u>	Water Content, %	19.69 ,		
Initial	Dry Density, pcf	107.6 ,		
	Saturation, %	96.45		
	Void Ratio	0.543		
Ur	nconfined Compressive Strength, psi	64.47		
Ur	ndrained Shear Strength, psi	32.24		
Tiı	me to Failure, min	7.001		
St	rain Rate, %/min	1		
М	easured Specific Gravity	2.66		
Li	quid Limit			
PI	astic Limit			
PI	asticity Index			
Fo	pilure Sketch			





Sy	rmbol	0		
Те	st No.	1		
	Diameter, in	2.765 ,		
	Height, in	6.104		
<u>ē</u>	Water Content, %	19.27		
Initial	Dry Density, pcf	109. ,		
	Saturation, %	95.79		
	Void Ratio	0.541		
Uı	nconfined Compressive Strength, psi	62.31		
U	ndrained Shear Strength, psi	31.16		
Ti	me to Failure, min	6.0023		
St	rain Rate, %/min	1		
М	easured Specific Gravity	2.69 '		
Li	quid Limit			
PI	astic Limit			
ΡI	asticity Index			
Fo	silure Sketch			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1



Service

Project: PLUM CREEK SITE #12
Location: TX
Project No.: 14-1229
Boring No.: 211.1

Sample Type: CORE

Description: ~17+25 cL AS 175' RIGHT, F14-326

Remarks: