

UNITED STATES GOVERNMENT

Memorandum

TO : P. M. Price, State Conservation Engineer,
SCS, Temple, Texas

DATE: July 18, 1967

FROM : Jack W. Adair, Head, E&WP Unit,
SCS, Fort Worth, Texas

Sumner Brune, Acting

SUBJECT: ENG - Soil Mechanics 22 - Texas, (WP-08) 2029
Plum Creek, Site 2
Hays County

ATTACHMENTS

- | | |
|---|----------|
| 1. Form SCS-372, Placement of Earth Fill Materials | 1 sheet |
| 2. Form SCS-354, Soil Mechanics Laboratory Data | 1 sheet |
| 3. Form SCS-352, Compaction and Penetration Resistance | 2 sheets |
| 4. Form SCS-35, Plan and Profiles for Geologic Investigations | 3 sheets |

INTERPRETATION OF DATA AND RECOMMENDATIONS

Foundation

This will be a 37' high class "a" dam on the Taylor formation. During the site investigation, foundation soils found consisted of about 7' of alluvial, slightly gravelly stiff CL and CH soils overlying 5' to 17' of residual shaly CH soils. Bedrock consists of hardness 1-2 shale at depths of 15' on the left abutment to 24' in the floodplain. No water table was found.

No foundation samples were submitted for testing. Relative permeabilities of the stiff CL and CH foundation soils are estimated to be low to very low.

At station 24 + 27 longtime settlement due to consolidation of the foundation is estimated to be 0.2'.

Centerline Cutoff

A cutoff similar to the one recommended by the Geologist and Project Engineer should be effective in controlling seepage in the foundation. The trench should extend below all dry-weather cracks.

Principal Spillway (Station 24 + 27 @ Dam)

Excavation for the conduit foundation need not extend deeper than is required to remove topsoil and roots and to provide grade for the pipe with its cradle. No joint gap problems are anticipated.

Borrow and Excavation

Compaction tests were performed on one composite sample from the emergency spillway and one from the borrow area. Densities obtained are recorded on Form SCS-352. The samples tested classify as SC and CH soils.

Refer to Form SCS-372 for recommended placement and compaction of earth fill materials.



Embankment Design (Maximum Embankment Height = 37')

Using shear strength values of $\phi = 1-1/2^\circ$ with $c = 1700$ psf for the embankment and $\phi = 9^\circ$ with $c = 1150$ psf for the foundation, MTS charts give factors of safety for slope stability of more than 2.0.

A standard embankment design with 2-1/2:1 slopes is recommended.

Residual settlement within the embankment is estimated to be 2% of the height of fill.

A total allowance of 1.0' should be added to the fill height for settlement of the foundation and embankment.

Drainage

No drainage measures are recommended.

cc: H. J. Behrens, Fort Worth, Texas
Harry W. Arfman, Fort Worth, Texas
James C. Evans, Waco, Texas
D. L Bidwell, Seguin, Texas - 2 copies
J. G. Hill, Corsicana, Texas

Prepared by: Gerald Tefft

Reviewed and Approved by:
Morris Reedy



MATERIALS TESTING REPORT U. S. DEPARTMENT of AGRICULTURE **SOIL CONSERVATION SERVICE** **COMPACTION AND PENETRATION RESISTANCE**

PROJECT and STATE Plum Creek Site 2 Texas

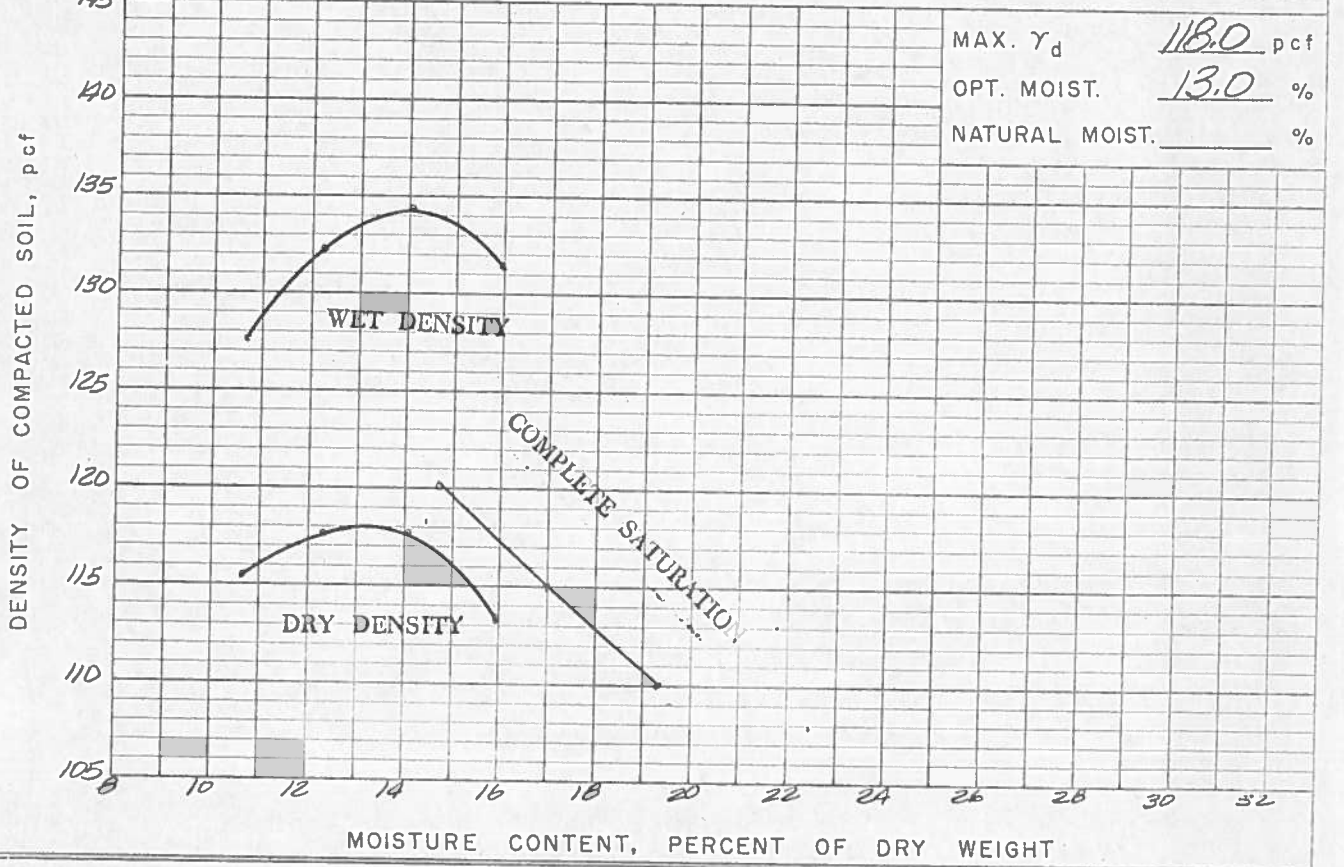
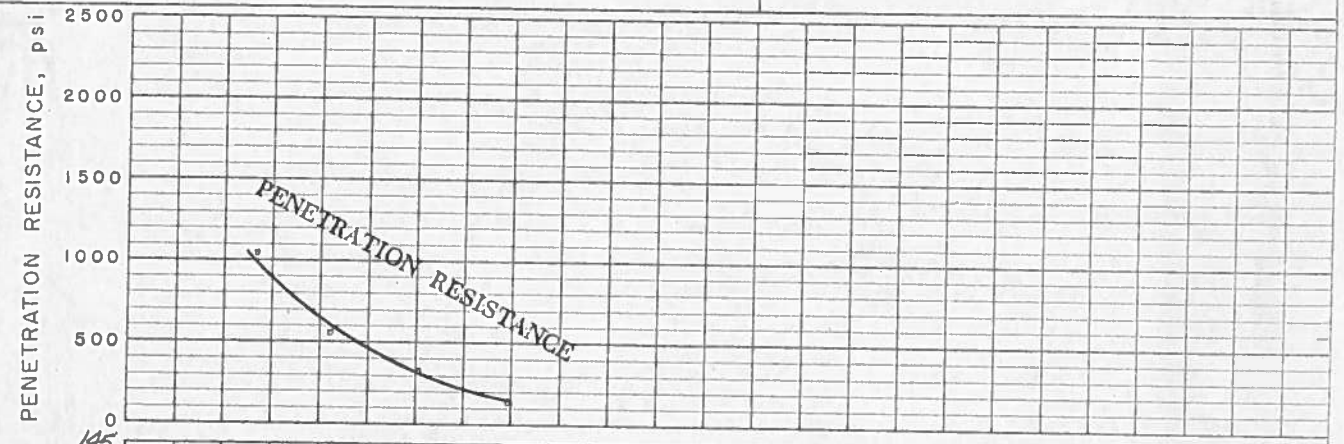
FIELD SAMPLE NO Comp. #1 LOCATION Emer. Spw DEPTH 2'-6"

GEOLOGIC ORIGIN _____ TESTED AT FWMIS APPROVED BY _____ DATE 6-7-67

CLASSIFICATION SC LL 28 PI 13 CURVE NO. 1 OF 2

MAX. PARTICLE SIZE INCLUDED IN TEST #4 " STD.(ASTM D-698) ; METHOD A

SPECIFIC GRAVITY (G_s) { MINUS NO. 4 2.68 MOD.(ASTM D-1557) ; METHOD -
PLUS NO. 4 - OTHER TEST (SEE REMARKS)



MAX. γ_d 118.0 pcf
OPT. MOIST. 13.0 %
NATURAL MOIST. _____ %

REMARKS

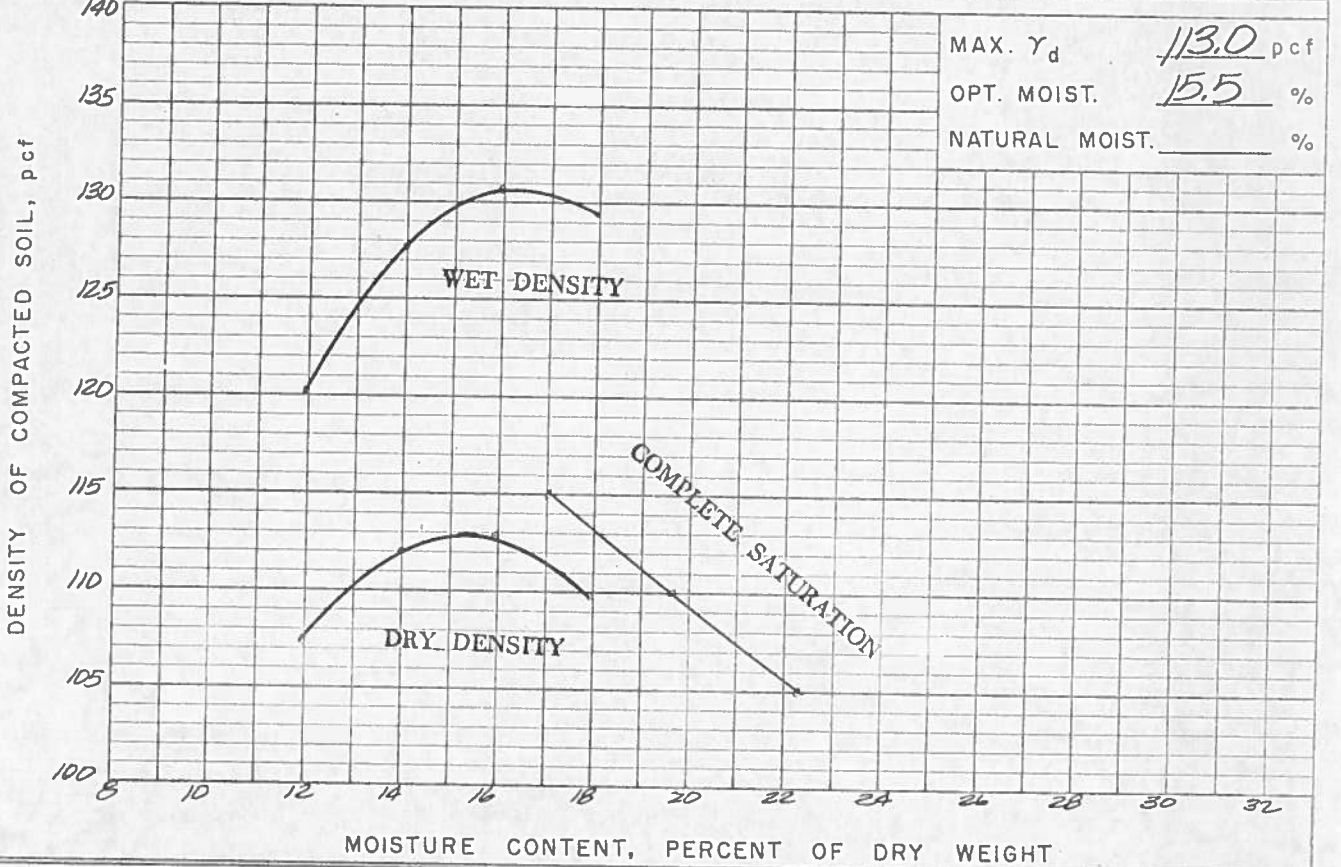
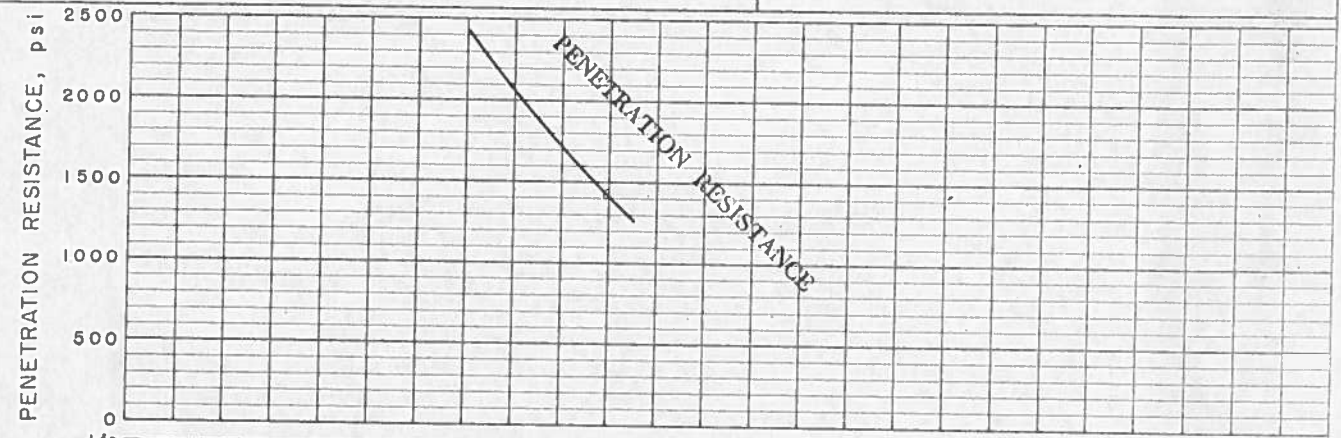
MATERIALS TESTING REPORT	U. S. DEPARTMENT of AGRICULTURE SOIL CONSERVATION SERVICE	COMPACTION AND PENETRATION RESISTANCE
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PROJECT and STATE Plum Creek Site 2 Texas

FIELD SAMPLE NO. <u>Comp #2</u>	LOCATION <u>Borrow</u>	DEPTH <u>1'-5'</u>
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GEOLOGIC ORIGIN	TESTED AT <u>FWMTS</u>	APPROVED BY	DATE <u>6-9-67</u>
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CLASSIFICATION <u>CH</u> LL <u>56</u> PI <u>34</u>	CURVE NO. <u>2</u> OF <u>2</u>
MAX. PARTICLE SIZE INCLUDED IN TEST <u>#4</u> "	STD. (ASTM D-698) <input type="checkbox"/> ; METHOD <u>-</u>
SPECIFIC GRAVITY (G_s)	MOD. (ASTM D-1557) <input checked="" type="checkbox"/> ; METHOD <u>A</u>
	OTHER TEST <input type="checkbox"/> (SEE REMARKS)
MINUS NO. 4 <u>2.69</u>	
PLUS NO. 4 <u>-</u>	



REMARKS

P. M. Price

U. S. DEPARTMENT OF AGRICULTURE
SOIL CONSERVATION SERVICE

TO: H. N. ~~Smith~~, State Conservationist,
SCS, Temple, Texas

Invoice No.: 371-67
Date: 7/18/67
Lab. Work Order No. 2051

FROM: Jack W. Adair, Head, E&WP Unit,
SCS, Fort Worth, Texas

SUBJECT: B&F - Records Management, Billing Invoice for Material Testing Service

Project and/or Site: Plum Creek, Site 2

The total amount shown is chargeable to State Order No.: TX-A153-67
Appropriation: 12x1067-TX-S(WP-08)2029

TESTING AND ENGINEERING

Sieve Analysis-Gravel	<u>2</u> @	\$3.00	<u>6.00</u>
Sieve Analysis-Sand	<u>2</u> @	2.75	<u>5.50</u>
Hydrometer Analysis, Total Salt and Dispersion	<u>3</u> @	6.00	<u>18.00</u>
Hydrometer Analysis and Dispersion	_____ @	5.75	_____
Dispersion	_____ @	5.75	_____
Total Soluble Salt	_____ @	1.50	_____
Atterberg Limits, (LL, PL, & PI)	<u>3</u> @	11.00	<u>33.00</u>
Shrinkage Limit, Lineal	_____ @	3.00	_____
Shrinkage Limit, Volumetric	_____ @	4.50	_____
Specific Gravity (G_s & G_m)	<u>3</u> @	2.50	<u>7.50</u>
Compaction D-698, Standard	<u>1</u> @	25.00	<u>25.00</u>
Compaction D-1557, Modified	<u>1</u> @	32.50	<u>32.50</u>
Core Opening and Preparation	_____ @	7.00	_____
Unit Dry Weight γ_d Undisturbed (Core or Clod)	_____ @	4.50	_____
Unit Dry Weight γ_d (Sedimentation Sample)	_____ @	2.50	_____
Moisture Content, Disturbed	_____ @	1.50	_____
Ignition Loss	_____ @	5.00	_____
Shear, Triaxial, 1.4" and 2.8"	_____ @	134.00	_____
Shear, Direct, 2" x 2"	_____ @	134.00	_____
Shear, Unconfined Compression	_____ @	45.00	_____
Consolidation	_____ @	110.00	_____
Permeability (Per Setup)	_____ @	22.00	_____
Sealing, Ponds and Reservoirs	_____ @	65.00	_____
Free Swell	_____ @	1.00	_____
Wet-Dry, Freeze-Thaw	_____ @	25.00	_____
	_____ @		_____
	_____ @		_____

Received 7/21/67

TOTAL \$127.50

Asst. State Conservation Engineer

Copies sent to: Fiscal Office Harry W. Arfman
P. M. Price James C. Evans